I-40/CC Westbound On-Ramp and Right-Turn Lane

CENE-486C Final Design Presentation

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- Finally, to all Professors who Offered their Guidance and Support Throughout the Duration of this Project.

Project Overview

- Client: Arizona Department of Transportation (ADOT)
- Location: I-40 and Country Club Drive Traffic Interchange (Flagstaff, AZ)
- Stakeholders:
 - ADOT
 - City of Flagstaff
 - General Public
- Final Product: 30% Design Concept Report



Project Location and Vicinity Map





Figure 1 (From Left to Right): Map of Arizona (NTS); Project Vicinity Map (NTS) [2][3]

Project Milestones (Major Tasks)

- Milestone 1: Process Survey Data
- **Milestone 2**: Input Existing Geometry
- **Milestone 3**: Create Construction Alignments
- Milestone 4: Create Existing Cross Sections
- Milestone 5: Complete Existing Runoff Calculations
- Milestone 6: Create Proposed Cross-Sections

- **Milestone 7**: Final Intersection Design
- Milestone 8: Final On-Ramp Design
- Milestone 9: Final Drainage Design
- **Milestone 10**: Capacity Analysis
- Milestone 11: Impacts Assessment

Processing Survey Data

- Contour Data was Downloaded as a .shp File from ArcGIS, and Processed within Civil 3D
- Parcel Information was Exported from GIS into Civil 3D



Figure 2: Existing Contour Data (NTS)

Input Existing Geometry

- Project Aerial was Georeferenced into Civil 3D
- Existing Geometry was Drawn in using Aerial, within Civil 3D
- Various Layers and Line Types were used for Existing Geometry



Figure 3: Existing Geometry (NTS)

Create Construction Alignments

- Country Club Drive CL Alignment
 - Stationing began at Intersection of US89
 - Placement Involved Offsetting Existing Edge of Pavement, Half the Existing Roadway Width
- I-40 On-Ramp Edge Alignment
 - Stationing Began at Intersection of Country club Drive
 - Placement Involved Offsetting Existing Edge of Pavement, Two Feet into Existing Roadway

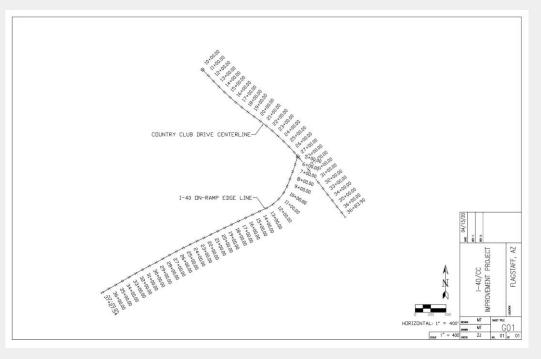


Figure 4: Alignment Geometry (NTS)

Existing Cross-Sections

- Obtained from As-Builts Provided by ADOT
- Includes
 - Pavement Structure
 - Lane Widths and Usage
 - Curb and Gutter Detail Callouts
 - Cross-Slopes

- Pavement Structure Included in Plan Set
- Existing Cross-Sectional Information was used for Proposed Cross-Sections

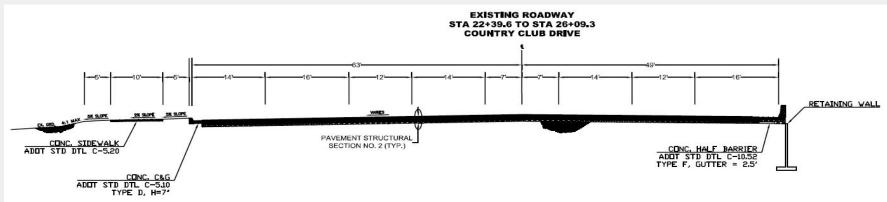


Figure 5: Country Club Drive Existing Cross-Section (NTS)

Existing Runoff Calculations

- Major Watershed was Delineated for Project Area
 - USGS Topo Maps
 - City Contour Data
 - Control Point was Box Culvert Crossing I-40 Westbound
- Peak Flows were Calculated Using National Stream Statistics (NSS)
 - Annual Precipitation (21 inches)
 - Watershed Area (2.02 square miles)
 - Region (Peak_Region_1_High_Elev_2014)
 - Peak Flow (50-yr) = 208 cfs
- Used flow from previous drainage report (more conservative flow)
 - Peak Flow (50-yr) = 1159 cfs [4]

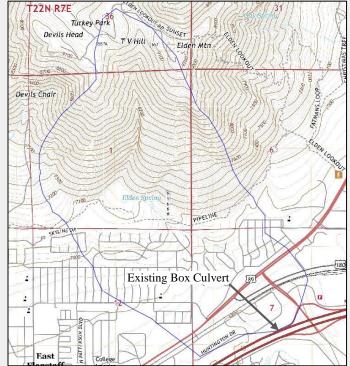


Figure 6: Project Watershed (NTS)

Existing Runoff Calculations Cont.

- Watershed was Delineated for Existing Catch Basin and 18" CMP
 - Rational Method
 - Bentley Flowmaster
 - Peak Flow (50-yr) = 33.2 cfs
- ADOT and COF Drainage Standards [5]
- Existing Infrastructure is able to Accommodate Existing Flows [6]

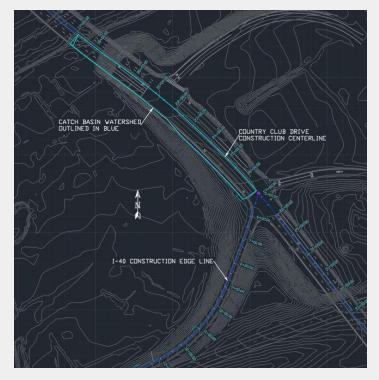


Figure 7: Catch Basin Watershed (NTS)

Create Proposed Cross-Sections

- 24" Saw Cut Offset from Existing Edge of Pavement
- Sheets TX01 TX03

- Match Existing Infrastructure
- Pavement Structure Included in

Plan Set

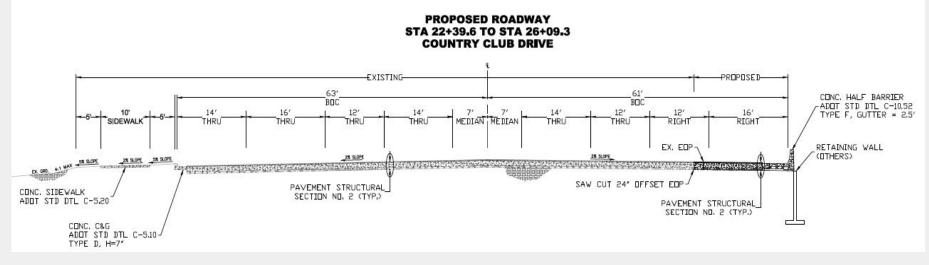
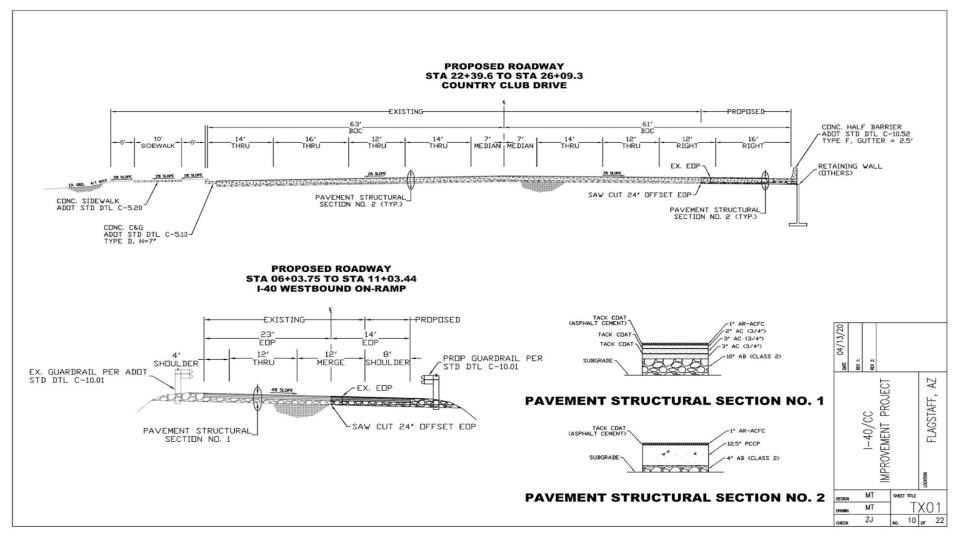


Figure 8: Proposed Cross-Sections



Detail Sheets

- Detail Sheets were Created for
 - Curb Types
 - Guardrail Construction
 - Concrete Barrier Construction
 - Wattle Placement for Erosion Control
 - Catch Basin Construction
- Sheets DT01-DT05 within Plan Set
 - Sheets 04-08

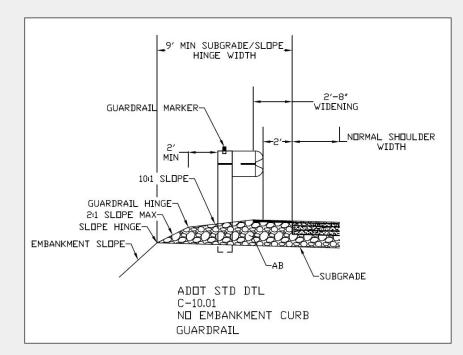
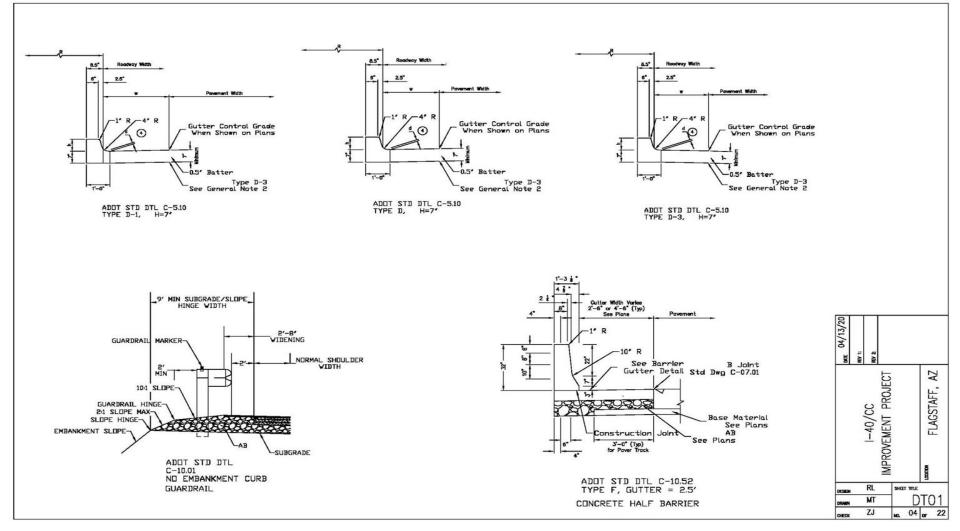


Figure 9: Example Guardrail Detail (NTS)



Final Intersection Design

- FHWA Lane Taper [7] recommendations
 - \circ Taper Length = 96'
 - Taper Slope = 8:1
- Right-Turn lane extends 512' to Existing Bridge Structure
- 12' Lane Width
- 4' Shoulder Width
- Sheets PV01 PV03

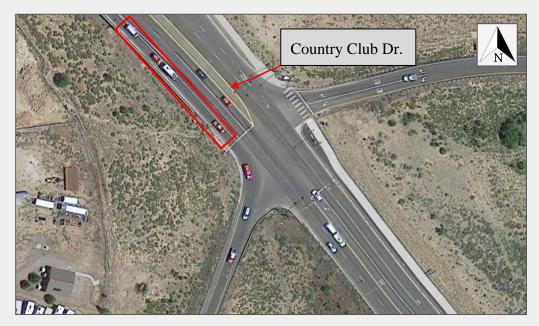


Figure 10: Intersection Plan View (NTS)

Final On-Ramp Design

- Federal Highway Administration (FHWA) Lane Taper Recommendations [7]
 - \circ Taper Length = 300'
 - \circ Taper Slope = 25:1
- Total Lane Length is 800'
- Match Existing Superelevation
- Guardrail Per ADOT STD DTL 10.01 [8]
- Sheets PV04 PV08

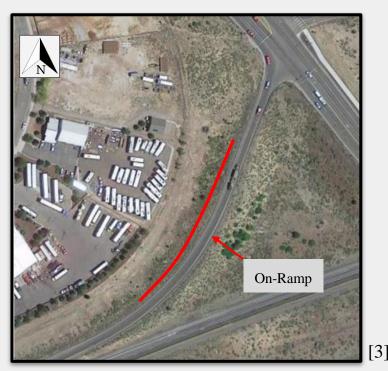
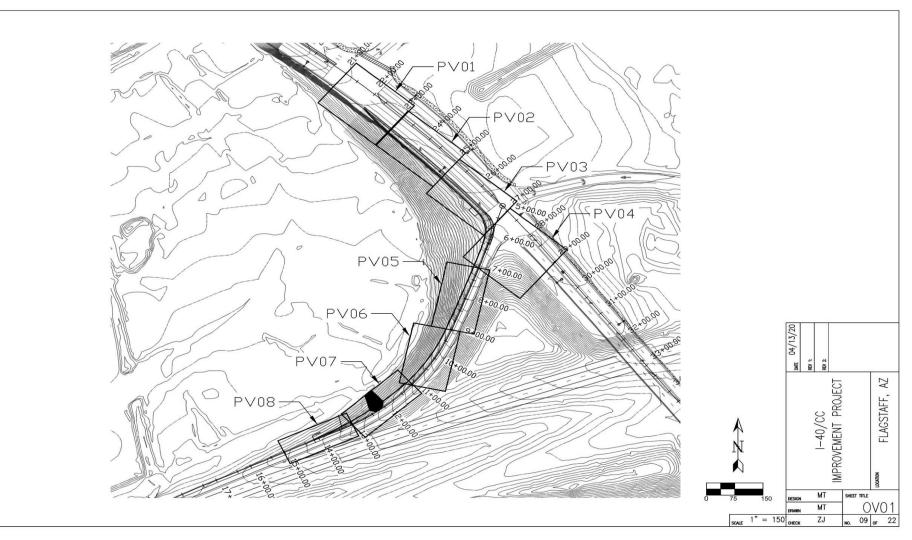
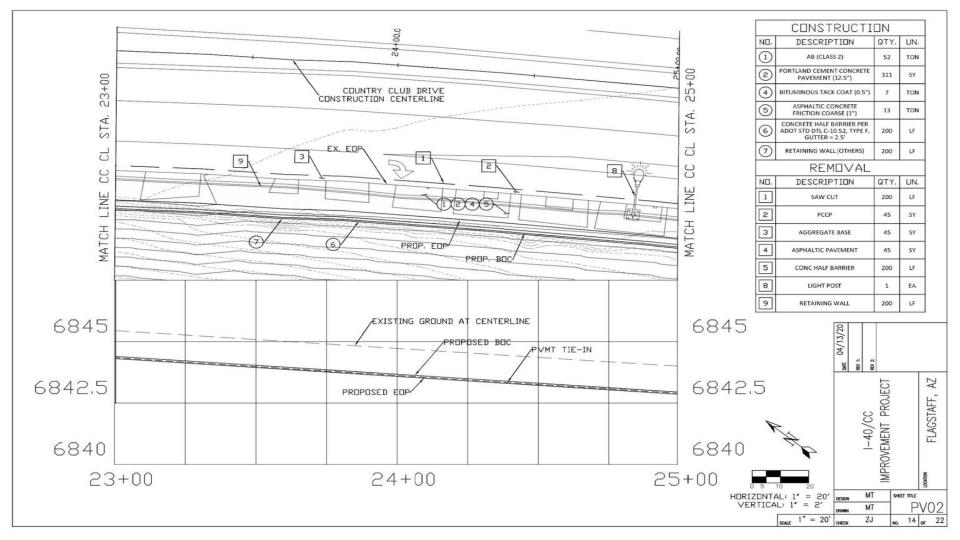


Figure 11: On-Ramp Plan View (NTS)

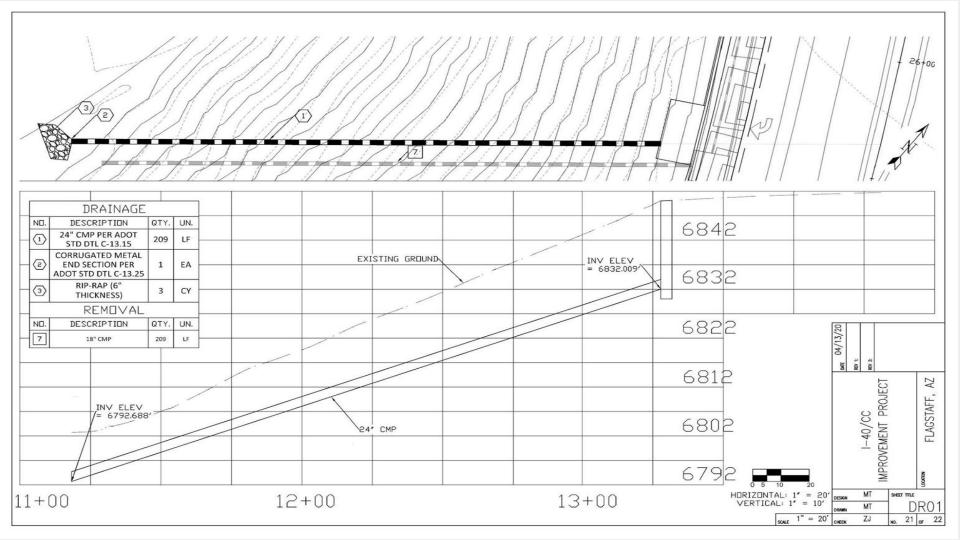




Final Drainage Design

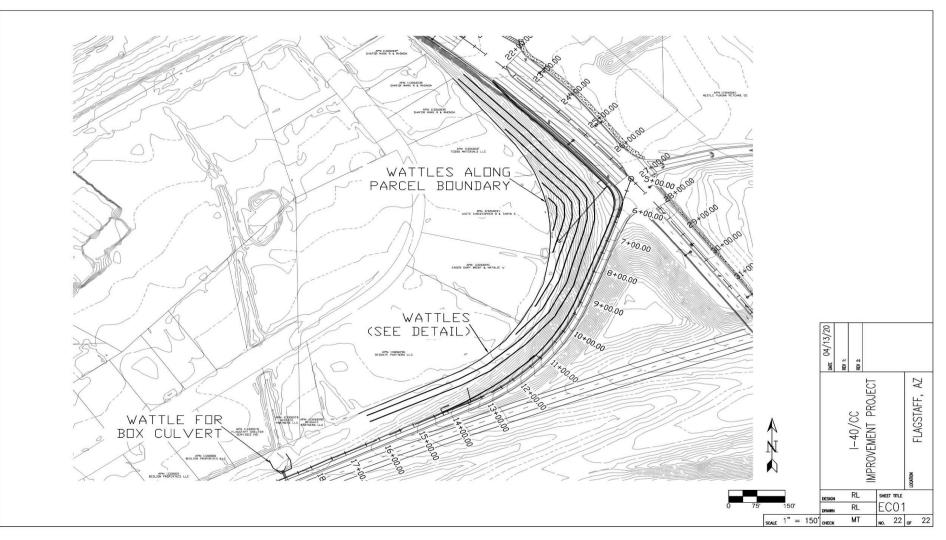
- Proposed Catch Basin Located at Intersection of I-40 Westbound On-Ramp and Country Club Dr
 - \circ Discharge (50-yr) = 35.35 cfs
 - \circ Efficiency = 80%
 - \circ Intercepted Flow = 28.28 cfs
 - \circ Bypass Flow = 7.07 cfs
 - Inlet Length = 22 ft

- Recommended Storm Pipe Up-Sizing from 18" to 24" due to Increased Runoff from Proposed Roadway [5] [6]
 - Proposed Flow (50-yr) = 29.2 cfs
 - Proposed Storm Pipe Capacity (24") = 66.97 cfs
 - Existing Storm Pipe Capacity (18") = 31.12 cfs
- Existing Storm Pipe must be Removed for Grading
- Storm Pipe Up-Sizing Recommended for Future Development



Erosion Control Plan

- Basic Erosion Control Plan Created Using ADOT Standard Details and Specifications
- Wattles Placed along Project for Erosion Control
- Horizontal Spacing = 20 ft maximum
- Detail Sheet Created for Wattle Placement Along Project
- Wattle Placement Designated at Storm Infrastructure Locations
 - Existing 10 ft x 6 ft Box Culvert Crossing I-40 Westbound at Southwestern End of Project
 - Outlet of Existing 18" Corrugated Metal Pipe
- Wattle Placement Designated at Parcel Boundary of Adjacent Properties



Capacity Analysis of Right-Turn Lane Group

Equation 1: Capacity

The Capacity of a Lane Group was Found using the Capacity Equation Below [9]:

$$c = Ns \cdot \frac{g}{C}$$

Where:

- c = Capacity of Lane Group
- N = Number of Lanes
- s = Adjusted Saturation Flow Rate
- g = Effective Green Time
- C = Cycle Length

- Capacity is the Amount of Vehicles that can pass a Point within a Lane per Hour.
- Capacity is Important due to its Direct Correlation to a Facility's Ability to Account for Various Traffic Conditions and Demands
- Only the Capacities of the Exclusive Right-Turn Lanes were Analyzed

Capacity Analysis of Right-Turn Lane Group Cont.

Equation 2: Adjusted Saturation Flow Rate:

$$s = s_0 f_w f_{HV_g} f_p f_{bb} f_a f_{LU} f_{LT} f_{RT} f_{Lpb} f_{Rpb} f_{wz} f_{ms} f_{sp}$$

- Adjusted Saturation Flow Rate is Amount of Vehicles that can Travel in an Individual Lane under Prevailing Conditions.
- Accounts for Factors such as Heavy Vehicles, Parking use, Work Zones, Lane Utilizations, and Public Transit Among Others
- Most Variables will have a Value of 1.0, having no Effect on the Adjusted Saturation Flow Rate

Capacity Analysis Calculation

- **Existing Conditions (1 RT Lane)** •
 - **Base Saturation Flow Rate** Ο
 - s = 1514 pc/hr/ln
 - Effective Green = $30 \sec (Assumed)$ Ο

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Cycle Length = 45 sec (Assumed) Ο

$$c = (1 \, lane)(1514 \, pc/hr/ln) \cdot \frac{30sec}{45sec}$$

$$c = 1009 \ veh/hr$$

- **Proposed Design (2 RT Lanes)**
 - **Base Saturation Flow Rate** 0
 - s = 1514 pc/hr/ln
 - Effective Green = $30 \sec (Assumed)$ Ο
 - Cycle Length = 45 sec (Assumed) Ο

$$c = (\underline{2 \ lane})(1514 \ pc/hr/ln) \cdot \frac{30sec}{45sec}$$

$$\frac{50sec}{45sec}$$

$$c=2018\; veh/hr$$

Conclusion

- Final Earthwork = 151,467.8 Net Cubic Yards of Fill
- Capacity of Intersection Doubled from Addition of New Lane
- Storm Water Pipe Attached to Catch Basin Up-Sized from 18" to 24"
- Recommendation to Increase Culvert Capacity Crossing I-40 Westbound to Three 8' x 6' RCBC
- Final On Ramp Design:
 - 4% Superelevation (Typical)
 - Taper Slope 25:1
- Final Intersection Design:
 - 2% Cross-Slope (Typical)
 - Lane Taper 8:1

- Total Length 800'
- Merge Lane Taper Length 300'
- Total Length 96'
- Additional Lane Width 12'

Impacts Assessment

Environmental Impacts

- Increase in Stormwater Pollution
- Increase in Impervious Surface Area
- Increase in Emissions

Social Impacts

- Increased Roadway Capacity
- Less Time on Roadway, more Time Getting to their Destination
- More Time with Family or Friends

Economic Impacts

- Less Time Waiting at Intersection, Quicker to get to Jobs or Businesses
- Less Waiting for Trucks to Deliver Goods

References

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- [2] "County Map of Arizona", *Mapsales.com*, 2020. [Online]. Available: *https://www.mapsales.com/county-wall-maps/arizona.aspx*. [Accessed: 12- Mar- 2020].
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- [5] City of Flagstaff Engineering Division (2009). *CITY OF FLAGSTAFF STORMWATER MANAGEMENT DESIGN MANUAL*. Flagstaff: City of Flagstaff, pp.3-1, 3-3, 3-4.
- [6] Arizona Department of Transportation (2020). *Roadway Design Guidelines*. Arizona Department of Transportation, pp.600-6, Appendix C.
- [7] Federal Highway Administration, 2013. *Guide For Highway Capacity And Operations Analysis Of Active Transportation And Demand Management Strategies*. United States Department of Transportation, p.55.
- [8] Arizona Department of Transportation, 2020. *Construction Standard Drawings*. Arizona Department of Transportation.
- [9] Highway Capacity Manual 2020. *Capacity Analysis Formula*, Appendix F.

Questions?